



**DESIGN ADJUSTMENT FACTORS AND THE ECONOMICAL
APPLICATION OF CONCRETE FLAT-SLABS WITH INTERNAL
SPHERICAL VOIDS IN SOUTH AFRICA**

by

CORNEILLE CHARLES MARAIS

**A dissertation submitted in the partial fulfilment of the
requirements for the degree of**

**MASTER OF ENGINEERING
(STRUCTURAL ENGINEERING)**

in the

**FACULTY OF ENGINEERING, BUILT ENVIRONMENT AND
INFORMATION TECHNOLOGY**

UNIVERSITY OF PRETORIA

AUGUST 2009

SUMMARY OF DISSERTATION

Design adjustment factors and the economical application of concrete flat-slabs with internal spherical voids in South Africa

by
Corneille Charles Marais

Supervisor: Dr John M Robberts
Co-Supervisor: Professor Ben van Rensburg
Department: Civil Engineering
University: University of Pretoria
Degree: Master of Engineering (Structural Engineering)

Keywords: spherical void formers, concrete flat slabs, shear strength, modelling, economy

Long span flat slab systems with internal spherical void formers have been used in Europe for a decade now. Cobiax® is the brand name of a successful system, recently introduced in South Africa. It is a bi-axial reinforced concrete flat slab system, with a grid of internal spherical void formers. The main advantage is the possibility of long spans due to the significant reduction in own weight, as well as the fast construction sequence with the use of flat slab formwork systems.

Design requirements of SANS 10100:2000 are affected. Vertical shear capacity is a concern due to loss of aggregate interlock. Research in Germany proved a factor of 0.55 to be a conservative shear resistance reduction factor for Cobiax slabs. Theoretical and preliminary laboratory South African research suggests that a greater factor of 0.85 might be used when considering the shear capacity of the steel cages. These cages' vertical legs also cross the cold joint caused by the two concrete pours required for Cobiax slabs, and proved to provide sufficient horizontal shear resistance if the correct cage diameters are used.

Laboratory tests in Germany supported by theoretical calculations further showed reduced deflections for Cobiax slabs. Although stiffness and own weight are reduced due to the voids, Cobiax slabs had smaller absolute deflections than solid slabs with the same thickness.

Cobiax research factors are safe to apply to SANS 10100-01:2000. The economy of Cobiax slabs was tested against that of coffer and post-tensioned slabs. Different span lengths and loads were considered. Based on 2007 material costs in South Africa, Cobiax slabs subject to the same loads and span lengths will be slightly more expensive than that of coffer slabs and post-tensioned slabs when considering only direct slab construction costs. Cobiax will be most appropriate where a flat soffit is required for high multi-storey buildings, requiring large spans with a light load application.

ACKNOWLEDGEMENT

This project report is based on a research project of Cobiax®. Permission to use the material is gratefully acknowledged. The opinions expressed are those of the old fitment and not necessarily represent the policy of Cobiax®.

I wish to express my appreciation to the following organisations and persons who made this project report possible:

a) The Cobiax organization for financial support and the University of Pretoria for the use of the laboratory facilities during the course of the study.

b) The following persons are gratefully acknowledged for their assistance during the course of the study.

- I.* Dr John Robberts
- II.* Prof Ben van Rensburg
- III.* Michael Stücklin
- IV.* Christian Roggenbuck
- V.* Chris Heesen
- VI.* Dévan Venter
- VII.* Jan Kotze
- VIII.* Johan Smit
- IX.* Kobus Nel
- X.* Ronélie Nel
- XI.* Oubaas & team
- XII.* Cobiax staff
- XIII.* Derrek Mostert
- XIV.* Herman Booysen
- XV.* Philip Pansengrow
- XVI.* Gerhard van den Berg
- XVII.* Pravesh Naidoo

c) Sandra my love.



TABLE OF CONTENTS

1.	INTRODUCTION	1-1
1.1	Background	1-1
1.2	Objectives of the study	1-3
1.3	Scope of the study	1-4
1.4	Methodology	1-5
1.5	Organisation of the report	1-8
2.	LITERATURE REVIEW	2-1
2.1	Introduction	2-1
2.2	Mechanism of shear resistance in reinforced concrete beams without shear reinforcement	2-1
2.3	Shear Resistance According to British Standards 8110	2-9
2.4	Shear Resistance According to SANS 10100-1:2000	2-12
2.5	Shear Resistance According to Eurocode 2	2-15
2.6	Cobiax Flat Slab Shear Resistance	2-17
2.7	Cobiax Flat Slab Deflection	2-20
2.8	Flat Plates	2-21
2.9	Elastic Theory Analysis of Slabs	2-21
2.10	Limit States and other Methods of Analysis for Slabs	2-22
2.11	Design Specifics for Flat Slabs	2-23
2.12	Analysis and Design of Flat Slab Structures	2-35
	2.12.1 Analysis of structure: equivalent frame method	2-35
	2.12.2 Analysis of structure: simplified method	2-36
	2.12.3 Lateral distribution of moments and reinforcement	2-37
	2.12.4 Wood and Armer Method for Concrete Slab Design	2-38
2.13	Design of Prestressed Concrete Flat Slabs	2-40
	2.13.1 Post-tensioning systems	2-40
	2.13.2 Design codes of practice	2-40
	2.13.3 Load Balancing	2-41
	2.13.4 Structural analysis of prestressed flat slabs	2-41
	2.13.5 Secondary effects	2-42
	2.13.6 Design Parameters	2-42
	2.13.7 Loading	2-44
	2.13.8 Lateral Loading	2-44
	2.13.9 Geometry of Tendons	2-44
	2.13.10 Prestress losses	2-46



TABLE OF CONTENTS – Continued

2.13.11	Serviceability Limit State	2-51
2.13.12	Ultimate Limit State Design	2-52
2.13.13	Minimum Un-tensioned Reinforcement	2-56
2.13.14	Crack control	2-57
2.14	Economy of Different Concrete Slab Systems	2-57
2.15	Conclusion	2-67
3.	EXPERIMENTAL WORK – SHEAR IN COBIAX SLABS	3-1
3.1	Introduction	3-1
3.2	Preparation and Experimental Setup	3-2
3.3	Observations	3-9
3.4	Results	3-10
3.5	Justification of Results	3-16
3.6	Conclusion	3-28
4.	ECONOMY OF THE COBIAX FLAT SLAB SYSTEM	4-1
4.1	Background	4-1
4.2	Main assumptions	4-2
4.3	Formwork	4-9
4.4	Cobiax slabs	4-9
4.5	Coffer slabs	4-19
4.6	Post-tensioned slabs	4-22
4.7	Results	4-26
4.8	Conclusion	4-43
5.	CONCLUSIONS AND RECOMMENDATIONS	5-1
6.	REFERENCES	6-1

APPENDICES

- Appendix A – Reinforcement provided
- Appendix B – Cobiax – Reinforcement required – Strand7
- Appendix C – Coffers – Reinforcement required – Strand7
- Appendix D – Post-tension – Reinforcement required – Strand7
- Appendix E – Formwork cost analysis
- Appendix F – Typical solid zones for Cobiax and coffer slabs – Strand7
- Appendix G – Cobiax - Punching shear reinforcement
- Appendix H – Shear contours for 620 mm thick Cobiax slab – Strand7
- Appendix I – Coffers – Punching shear reinforcement
- Appendix J – Post-tension slabs – Punching shear reinforcement
- Appendix K – Post-tension slabs – Cable design

LIST OF TABLES

2.11.1	Basic span/effective depth ratios for rectangular beams: Span/250	2-32
2.11.2	Modification factors for compression reinforcement	2-34
2.12.1	Bending moments and shear force coefficients for flat slabs of three or more equal spans	2-37
2.12.2	Distribution of moments in panels of flat slabs designed as continuous frames	2-38
2.13.1	Allowable average stresses in flat slabs, (two-way spanning), analysed using the equivalent frame method – Report No. 43	2-52
2.14.1	Waffle Slab Design	2-63
2.14.2	Unbonded Post-tensioned Flat Slab Design	2-66
3.2.1	$\frac{a_v}{d}$ ratios	3-4
3.2.2	Comparison between moment failure loads and shear failure loads based purely on design values	3-7
3.4.1	Beams tested and results obtained	3-10
3.5.1	Concrete test cubes and beams results	3-17
3.5.2	Steel test results	3-17
3.5.3	Comparison between moment failure loads and shear failure loads based on actual values	3-17

LIST OF TABLES - Continued

3.5.4	Comparison between test results and values predicted by SANS 10100	3-19
3.5.5	Comparison between test results and values predicted by Eurocode 2	3-20
3.5.6	Shear resistance of cages	3-25
3.5.7	Rough indication of the cages' shear capacity	3-27
4.1	Deflections	4-6
4.2	Cobiax stiffness reduction factors	4-14
4.3	Coffer stiffness reduction factors	4-21
4.4	Calculation of post-tension cost per kg	4-23
4.5	Post-tension content	4-24
4.6	Material Cost 2007	4-26
4.7	Cobiax, Coffer & Post-tensioned Slab Cost Comparison - Light Load	4-27
4.8	Cobiax, Coffer & Post-tensioned Slabs Cost Comparison - Medium Load	4-27
4.9	Cobiax, Coffer & Post-tensioned Slabs Cost Comparison - Heavy Load	4-28

LIST OF FIGURES

2.2.1	Trajectories of principle stresses in a homogenous isotropic beam	2-2
2.2.2	Equilibrium requirements in the shear span of a beam	2-4
2.2.3	Crack patterns in beams tested by Leonhardt and Walther	2-5
2.2.4	Moments and shears at failure plotted against shear span to depth ratio	2-6
2.2.5	Shear capacity of beams with varying reinforcement ratios	2-8
2.3.1	Experimental results and capacities predicted by BS 8110	2-11
2.6.1	Typical illustration of a Cobiax slab and its components	2-18
2.6.2	Mean width for cross section of Bubbledeck	2-19
2.11.1	Division of flat slab panels into column and middle strips	2-24
2.11.2	Shear at slab internal column connection	2-26
2.11.3	Punching shear zones	2-30
2.12.1	Equilibrium of a reinforced concrete membrane	2-38
2.13.1	Tendon equivalent loads for a typical tendon profile	2-41
2.13.2	Tendon geometry for a typical tendon profile	2-44
2.13.3	Drying shrinkage of normal-density concrete	2-49
2.13.4	Effects of relative humidity, age of concrete at loading and section thickness upon creep factor	2-50

LIST OF FIGURES - Continued

2.13.5	Determination of λ for use in Eq. 2.13.10	2-53
2.14.1	Preliminary Cobiax Design Chart	2-59
2.14.2	Waffle Slab Design Chart	2-62
2.14.3	Unbonded Post-tensioned Flat Slab Design Chart	2-65
3.2.1	Cross section of a 280 mm thick Cobiax sample	3-3
3.2.2	Experimental setup	3-6
3.2.3	Predicted moment failure and shear failure loads based on design values	3-8
3.4.1	Failure stress of all beams compared to SANS 10100 characteristic shear capacity	3-11
3.4.2	Load-deflection response of solid slabs	3-12
3.4.3	Shear capacity of 280mm solid slabs compared to characteristic predicted values	3-13
3.4.4	Load-deflection response of 280 mm slabs with 3Y16's	3-13
3.4.5	Load-deflection response of 280 mm slabs with 4Y16's	3-14
3.4.6	Load-deflection response of 280 mm slabs with 5Y16's	3-14
3.4.7	Load-deflection response of 295 mm Cobiax slabs	3-15
3.4.8	Load-deflection response of 310 mm Cobiax slabs	3-16
3.5.1	Predicted moment failure and shear failure loads based on design values	3-18
3.5.2	Comparison between predicted shear failure values and test results (SANS 10100)	3-21
3.5.3	Comparison between predicted shear failure values and test results (EC 2)	3-21
3.5.4	Cage spacing and dimensions	3-22
3.5.5	Design shear capacity of Cobiax slabs	3-24
4.1	Cobiax and Coffe slab solid zone layouts	4-11
4.2	Cobiax stiffness calculation method	4-13
4.2B	Coffe system	4-21
4.3	Post-tension content	4-25
4.4	Concrete content of slab systems [SDL=0.5kPa & ADL=2.0kPa]	4-31
4.5	Reinforcement content of slab systems [SDL=0.5kPa & ADL=2.0kPa]	4-32
4.6	Slab thickness of slab systems [SDL=0.5kPa & ADL=2.0kPa]	4-33
4.7	Cost of slab systems [SDL=0.5kPa & ADL=2.0kPa]	4-34
4.8	Concrete content of slab systems [SDL=2.5kPa & ADL=2.5kPa]	4-35
4.9	Reinforcement content of slab systems [SDL=2.5kPa & ADL=2.5kPa]	4-36
4.10	Slab thickness of slab systems [SDL=2.5kPa & ADL=2.5kPa]	4-37
4.11	Cost of slab systems [SDL=2.5kPa & ADL=2.5kPa]	4-38

LIST OF FIGURES - Continued

4.12	Concrete content of slab systems [SDL=5.0kPa & ADL=5.0kPa]	4-39
4.13	Reinforcement content of slab systems [SDL=5.0kPa & ADL=5.0kPa]	4-40
4.14	Slab thickness of slab systems [SDL=5.0kPa & ADL=5.0kPa]	4-41
4.15	Cost of slab systems [SDL=5.0kPa & ADL=5.0kPa]	4-42

LIST OF PHOTOS

2.6.1	Flat soffit of a 16m span Cobiax flat-slab, Freistadt, Germany	2-17
3.2.1	Experimental setup	3-4
3.3.1	Observed crack patterns at failure	3-9