



Figure A3.1

Vertical stress component along profile line with 2mand 8m-thick shale layer and 70° and 90° slope angle, in the undulated strata formation with 15° inclination



Figure A3.2

Vertical stress difference along profile line with 2mand 8m-thick shale layer and 70° and 90° slope angle, in the undulated strata formation with 5° inclination



Figure A3.3

Resultant horizontal stress component of the slope with 2m- and 8m-thick embedded shale layer and 70° and 90°-slope in the undulated strata formation with 15° layer inclinations



Figure A3.4

Horizontal stress component difference ($\Delta \sigma_{XX}$) of the slope with 2m- and 8m-thick embedded shale layer and 70[°] and 90[°] slope in the undulated strata formation with 5[°] layer inclinations



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Shear stress component of the profile with 2m- and 8mthick embedded shale layer, 70° and 90° -slope angle in the undulated strata formation (15° inclination) in the model









Vertical stress component difference ($\Delta\sigma_{YY}$) of the pillars with safety factor of 2.2 for the profiles with

slope angles 70° and 90° and the undulated strata formation (layer inclinations 5° and 15°) in the model



Figure A3.8

 $\Delta\sigma_{XX}$ of the pillars with a pillar safety factor of 2.2 of profiles with 70[°] and 90[°] slope angle and undulated strata formation (5[°] and 15[°] layer inclination) in the model



Figure A3.9

Shear stress component difference $(\Delta \sigma_{XY})$ of the pillars with a pillar safety factor of 2.2 in the profiles with 70° and 90° slope angle and undulated strata formation (5° and 15° layer inclination) in the model



Figure A3.10

Inclination of the principal stress direction angles along the profile lines at 15m, 20m and 25m depth (Figure 5.3) in the homogeneous sandstone slope profile with 70⁰ slope angle and different layer inclination







Inclination of the principal stress direction inclination angles along the profile line at 15m depth (Figure 5.3) in the profile with different embedded shale layer thickness at the undulated strata formation with: a) 5⁰ layer inclination and b) 15⁰ layer inclination







Inclination of the principal stress direction inclination angles along the part of profile line at 20m depth (Figure 5.3) in the profile with different embedded shale layer thickness at the undulated strata formation with: a) 5⁰ layer inclination and b) 15⁰ layer inclination









Inclination of the principal stress direction angles along the profile line at 25m depth in the profile with different embedded shale layer thickness at the undulated strata formation with: a) 5⁰ layer inclination and b) 15⁰ layer inclination

APPENDIX 4. SLOPE STABILITY CALCULATIONS

Table A4.1 *Example 1b*: Safety factor calculations along the bottom contact

Step	No	Parameter	Value	Equation No
		Inner shear failure surface	(ISFS)	
Step	1	Average friction angle, deg	26	5.2
Step	2	Inclination angle of the ISFS, deg	45	-
Step	3	Average cohesion of the ISFS, MN/m^2	0.307	5.3
Step	4	Length of the ISFS, m	16.69	-
Step	5	Inner side of the active block construction	_	_
		Outer shear failure surface	(OSFS)	-
Step	6	Average friction angle, deg	28	5.2
Step	7	Inclination angle of the OSFS, deg	45	_
Step	8	Average cohesion of the OSFS, $M\!N/m^2$	0.344	5.3
Step	9	Length of the ISFS, m	17.28	-
Step	10	Outer side of the active block construction	-	-
		General parameters		
Step	11	Frictional zone load, MN/m^2	11.527	5.4
Step	12	Frictional zone length, m	29.0	_
Step	13	Frictional zone inclination angle, deg	16	-
Step	14	Cohesive zone load, MN/m^2	2.084	5.4
Step	15	Cohesive zone length, m	12.0	-
Step	16	Cohesive zone inclination angle, deg	12	-
Step	17	Active block load, MN	12.486	5.4
Step	18	Active block wedge angle, deg	10	-
Step	19	Passive block reaction forces to the inner failure surface, MN	-0.567	5.28

Appendix	4.	Slope	stability	calculations
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Step 20	Pore water pressure calculations		5.24- 5.27
Step 21	Safety factor of the inner shear failure surface (condition of existence)	0.914	5.32
Step 22	Reaction force along the outer shear failure surface, MN	5.619	5.33
Step 23	Outer shear failure surface safety factor	0.709	5.35
Step 24	Basal shear surface safety factor	0.750	5.36
Step 25	Slope stability safety factor	0.777	5.37

Table A4.2 Example 2a: Safety factor calculations along

the	upper	contact	surface	before	initial	failure
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Step No	Parameter	Value	Equation No
	Inner shear failure surface	(ISFS)	
Step 1	Average friction angle, deg	28	5.2
Step 2	Inclination angle of the ISFS, deg	45	_
Step 3	Average cohesion of the ISFS, $\rm MN/m^2$	0.238	5.3
Step 4	Length of the ISFS, m	9.24	-
Step 5	Inner side of the active block construction	-	_
	Outer shear failure surface	(OSFS)	
Step 6	Average friction angle, deg	28	5.2
Step 7	Inclination angle of the OSFS, deg	45	-
Step 8	Average cohesion of the OSFS, $\rm MN/m^2$	0.228	5.3
Step 9	Length of the ISFS, m	11.55	-
Step 10	Outer side of the active block construction	-	-
	General parameters		
Step 11	Frictional zone load, MN/m^2	4.524	5.4
Step 12	Frictional zone length, m	27	_

Step 13	Frictional zone inclination angle, deg	12	-
Step 14	Cohesive zone load, MN/m^2	0.964	5.4
Step 15	Cohesive zone length, m	10.0	-
Step 16	Cohesive zone inclination angle, deg	10	-
Step 17	Active block load, MN	9.194	5.4
Step 18	Active block wedge angle, deg	8	-
Step 19	Passive block reaction forces to the inner failure surface, MN	0.674	5.28
Step 20	Pore water pressure calculations	Above the phreatic level	
Step 21	Safety factor of the inner shear failure surface (condition of existence)	1.010	5.32
Step 22	Reaction force along the outer shear failure surface, MN	1.923	5.33
Step 23	Outer shear failure surface safety factor	0.955	5.35
Step 24	Basal shear surface safety factor	1.108	5.36
Step 25	Slope stability safety factor	1.062	5.37

Table A4.3 *Example 2b*: Safety factor calculations along the bottom contact surface before initial failure

Step No	Parameter	Value	Equation No
	Inner shear failure surface	(ISFS)	
Step 1	Average friction angle, deg	26	5.2
Step 2	Inclination angle of the ISFS, deg	45	-
Step 3	Average cohesion of the ISFS, $M\!N/m^2$	0.323	5.3
Step 4	Length of the ISFS, m	16.17	-
Step 5	Inner side of the active block construction	-	-

	Outer shear failure surface	(OSFS)	
Step 6	Average friction angle, deg	26	5.2
Step 7	Inclination angle of the OSFS, deg	45	-
Step 8	Average cohesion of the OSFS, $\ensuremath{\text{MN}/\text{m}^2}$	0.326	5.3
Step 9	Length of the ISFS, m	19.25	_
Step 10	Outer side of the active block construction	-	-
	General parameters		
Step 11	Frictional zone load, MN/m^2	10.704	5.4
Step 12	Frictional zone length, m	29	-
Step 13	Frictional zone inclination angle, deg	14	-
Step 14	Cohesive zone load, MN/m^2	1.614	5.4
Step 15	Cohesive zone length, m	10.0	-
Step 16	Cohesive zone inclination angle, deg	12	-
Step 17	Active block load, MN	11.679	5.4
Step 18	Active block wedge angle, deg	10	-
Step 19	Passive block reaction forces to the inner failure surface, MN	-0.243	5.28
Step 20	Pore water pressure calculations		5.24- 5.27
Step 21	Safety factor of the inner shear failure surface (condition of existence)	1.036	5.32
Step 22	Reaction force along the outer shear failure surface, MN	5.064	5.33
Step 23	Outer shear failure surface safety factor	0.927	5.35
Step 24	Basal shear surface safety factor	0.917	5.36
Step 25	Slope stability safety factor	0.945	5.37

Table A4.4 *Example 2c*: Safety factor calculations along the upper contact surface before major collapse

Step	No	Parameter	Value	Equation No			
	Inner shear failure surface (ISFS)						
Step	1	Average friction angle, deg	28	5.2			
Step	2	Inclination angle of the ISFS, deg	45	-			
Step	3	Average cohesion of the ISFS, $\mathrm{MN/m}^2$	0.249	5.3			
Step	4	Length of the ISFS, m	8.47	_			
Step	5	Inner side of the active block construction	-	-			
		Outer shear failure surface	(OSFS)				
Step	6	Average friction angle, deg	28	5.2			
Step	7	Inclination angle of the OSFS, deg	45	-			
Step	8	Average cohesion of the OSFS, $M\!N/m^2$	0.269	5.3			
Step	9	Length of the OSFS, m	10.40	_			
Step	10	Outer side of the active block construction	-	-			
		General parameters					
Step	11	Frictional zone load, MN/m^2	5.821	5.4			
Step	12	Frictional zone length, m	32	_			
Step	13	Frictional zone inclination angle, deg	11	-			
Step	14	Cohesive zone load, MN/m^2	0.882	5.4			
Step	15	Cohesive zone length, m	7	_			
Step	16	Cohesive zone inclination angle, deg	8	-			
Step	17	Active block load, MN	5.093	5.4			
Step	18	Active block wedge angle, deg	7	_			
Step	19	Passive block reaction forces to the inner failure surface, MN	0.392	5.28			
Step	20	Pore water pressure calculations	Abov	ve the ic level			

Step 21	Safety factor of the inner shear failure surface (condition of existence)	1.149	5.32
Step 22	Reaction force along the outer shear failure surface, MN	1.674	5.33
Step 23	Outer shear failure surface safety factor	0.935	5.35
Step 24	Basal shear surface safety factor	1.318	5.36
Step 25	Slope stability safety factor	1.224	5.37

Table A4.5 Example 2d: Safety factor calculations along

the bottom contact surface before major collapse

Step	No	Parameter	Value	Equation No	
		Inner shear failure surface	(ISFS)		
Step	1	Average friction angle, deg	26	5.2	
Step	2	Inclination angle of the ISFS, deg	45	-	
Step	3	Average cohesion of the ISFS, MN/m^2	0.315	5.3	
Step	4	Length of the ISFS, m	1615	_	
Step	5	Inner side of the active block construction	_	_	
		Outer shear failure surface	(OSFS)		
Step	6	Average friction angle, deg	26	5.2	
Step	7	Inclination angle of the OSFS, deg	45	_	
Step	8	Average cohesion of the OSFS, MN/m^2	0.322	5.3	
Step	9	Length of the ISFS, m	19.23	_	
Step	10	Outer side of the active block construction	-	-	
	General parameters				
Step	11	Frictional zone load, MN/m^2	13.589	5.4	
Step	12	Frictional zone length, m	34	-	
Step	13	Frictional zone inclination angle, deg	14	_	

Appendix	4.	Slope	stability	calculations
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Step 14	Cohesive zone load, MN/m^2	1.674	5.4
Step 15	Cohesive zone length, m	11	_
Step 16	Cohesive zone inclination angle, deg	11	_
Step 17	Active block load, MN	10.374	5.4
Step 18	Active block wedge angle, deg	9	_
Step 19	Passive block reaction forces to the inner failure surface, MN	-0.420	5.28
Step 20	Pore water pressure calculations		5.24- 5.27
Step 21	Safety factor of the inner shear failure surface (condition of existence)	1.161	5.32
Step 22	Reaction force along the outer shear failure surface, MN	4.619	5.33
Step 23	Outer shear failure surface safety factor	0.884	5.35
Step 24	Basal shear surface safety factor	0.883	5.36
Step 25	Slope stability safety factor	0.939	5.37

Table A4.6 Example T-1: Safety factor calculations along the upper contact plane of the test profile $(5^{\circ}$ flatter slope angle compared to the slope angle of the profile before major collapse)

Step No	Parameter	Value	Equation No
	Inner shear failure surface	(ISFS)	
Step 1	Average friction angle, deg	28	5.2
Step 2	Inclination angle of the ISFS, deg	45	-
Step 3	Average cohesion of the ISFS, $\ensuremath{\text{MN}/\text{m}^2}$	0.260	5.3
Step 4	Length of the ISFS, m	10.40	-
Step 5	Inner side of the active block construction	_	_

Outer shear failure surface (OSFS)				
Step 6	Average friction angle, deg	28	5.2	
Step 7	Inclination angle of the OSFS, deg	45	-	
Step 8	Average cohesion of the OSFS, $\ensuremath{\text{MN}/\text{m}^2}$	0.250	5.3	
Step 9	Length of the ISFS, m	13.48	-	
Step 10	Outer side of the active block construction	-	Ι	
	General parameters			
Step 11	Frictional zone load, MN/m^2	6.195	5.4	
Step 12	Frictional zone length, m	41	_	
Step 13	Frictional zone inclination angle, deg	11	-	
Step 14	Cohesive zone load, MN/m^2	4.886	5.4	
Step 15	Cohesive zone length, m	19	_	
Step 16	Cohesive zone inclination angle, deg	6	-	
Step 17	Active block load, MN	6.425	5.4	
Step 18	Active block wedge angle, deg	3	_	
Step 19	Passive block reaction forces to the inner failure surface, MN	1.742	5.28	
Step 20	Pore water pressure calculations	Abov phreat	ve the ic level	
Step 21	Safety factor of the inner shear failure surface (condition of existence)	1.710	5.32	
Step 22	Reaction force along the outer shear failure surface, MN	0.852	5.33	
Step 23	Outer shear failure surface safety factor	1.222	5.35	
Step 24	Basal shear surface safety factor	2.029	5.36	
Step 25	Slope stability safety factor	1.860	5.37	

Table A4.7 Example T-2: Safety factor calculations along the bottom contact plane of the test profile $(5^{\circ}$ flatter slope angle compared to the slope angle of the profile before major collapse)

Step	No	Parameter	Value	Equation No
		Inner shear failure surface	(ISFS)	
Step	1	Average friction angle, deg	26	5.2
Step	2	Inclination angle of the ISFS, deg	45	_
Step	3	Average cohesion of the ISFS, $\mathrm{MN/m}^2$	0.321	5.3
Step	4	Length of the ISFS, m	17.31	_
Step	5	Inner side of the active block construction	-	-
		Outer shear failure surface	(OSFS)	
Step	6	Average friction angle, deg	26	5.2
Step	7	Inclination angle of the OSFS, deg	45	-
Step	8	Average cohesion of the OSFS, MN/m^2	0.302	5.3
Step	9	Length of the ISFS, m	18.86	-
Step	10	Outer side of the active block construction	-	_
		General parameters		
Step	11	Frictional zone load, MN/m^2	14.547	5.4
Step	12	Frictional zone length, m	43	_
Step	13	Frictional zone inclination angle, deg	12	_
Step	14	Cohesive zone load, MN/m^2	7.374	5.4
Step	15	Cohesive zone length, m	22.0	-
Step	16	Cohesive zone inclination angle, deg	б	_
Step	17	Active block load, MN	10.521	5.4
Step	18	Active block wedge angle, deg	4	_
Step	19	Passive block reaction forces to the inner failure surface, MN	1.463	5.28

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Step 20	Pore water pressure calculations		5.24- 5.27
Step 21	Safety factor of the inner shear failure surface (condition of existence)	1.658	5.32
Step 22	Reaction force along the outer shear failure surface, MN	2.688	5.33
Step 23	Outer shear failure surface safety factor	1.527	5.35
Step 24	Basal shear surface safety factor	1.378	5.36
Step 25	Slope stability safety factor	1.454	5.37

APPENDIX 5. SAFETY FACTORS FOR OPENCAST MINING

UNITED STATES

1. FEDERAL REGISTER - 1977

		Minimum FOS
I.	End of construction	1.3
II.	Partial pool with steady seepage saturation	1.5
III.	Steady seepage from spillway or decent crest	1.5
IV.	Earthquake (cases II and III with seismic loading)	1.0

2. D'APPOLONIA CONSULTING ENGINEERS, Inc. - 1975

Suggested minimum FOS with hazard potential

	High	Moderate	Low
Designs based on shear strength parameters measured in laboratory	1.5	1.4	1.3
Designs that consider maximum seismic acceleration expected at the site	1.2	1.1	1.0

CANADA

MINES BRANCH - 1972

Suggested minimum FOS with hazard potential

		High	Low
1.	Design is based on peak shear strength parameters	1.5	1.3
2.	Design is based on residual shear strength parameters	1.3	1.2
3.	For horizontal sliding on the base of dyke in seismic areas assuming shear strength of fine refuse in impoundment reduced to zero	1.3	1.3

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